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WHEELER POND DAM

CT 00239

PHASE 1 INSPECTION REPORT
NATIONAL DAM INSPECTION PROGRAM

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DEPARTMENT OF THE ARMY
MEW ENGLAND DIVISION, CORPS OF ENGINEERS"
WALTHAM, MASS.

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SEPTEMBER 1978

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DAMS, INSPECTION, DAM SAFETY,

Thames River Basin Montville, Conn.

Wheeler Pond Dam is a masonry structure constructed in the early 1800's. The dam has a maximum height of 20.0 ft. and is approx. 54.0 ft. long. Based upon the visual inspection at the site, the lack of engineering back-up data available, and no operational or maintenance evidence, there are areas of concern which must be corrected to assure the long term performance of this dam. The dam is considered to be in fair condition with the following visible signs of concern: the inoperable lower gate, poor condition of the mill intake and the obstructed downstream channel.



### DEPARTMENT OF THE ARMY

NEW ENGLAND DIVISION. CORPS OF ENGINEERS
424 TRAPELO ROAD
WALTHAM, MASSACHUSETTS 02154

REPLY TO ATTENTION OF:

NEDED

OCT 2 6 1978

Honorable Ella T. Grasso Governor of the State of Connecticut State Capitol Hartford, Connecticut 06115

Dear Governor Grasso:

I am forwarding to you a copy of the Wheeler Pond Dam Phase I Inspection Report, which was prepared under the National Program for Inspection of Non-Federal Dams. This report is presented for your use and is based upon a visual inspection, a review of the past performance and a brief hydrological study of the dam. A brief assessment is included at the beginning of the report. I have approved the report and support the findings and recommendations described in Section 7 and ask that you keep me informed of the actions taken to implement them. This follow-up action is a vitally important part of this program.

A copy of this report has been forwarded to the Department of Environmental Protection, the cooperating agency for the State of Connecticut. In addition, a copy of the report has also been furnished the owner, Mr. T. Wisniewski, 996 Norwich Turnpike, Uncasville, Connecticut 06382.

Copies of this report will be made available to the public, upon request, by this office under the Freedom of Information Act. In the case of this report the release date will be thirty days from the date of this letter.

I wish to take this opportunity to thank you and the Department of Environmental Protection for your cooperation in carrying out this program.

Sincerely yours,

Incl As stated JOHN P. CHANDLER Colonel, Corps of Engineers Division Engineer

# WHEELER POND DAM CT 00239

THAMES RIVER BASIN MONTVILLE, CONNECTICUT

# PHASE I INSPECTION REPORT NATIONAL DAM INSPECTION PROGRAM

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# NATIONAL DAM INSPECTION PROGRAM PHASE 1 - INSPECTION REPORT

IDENTIFICATION NO.: CT 00239

NAME OF DAM: WHEELER POND DAM

TOWN: MONTVILLE

COUNTY AND STATE: NEW LONDON COUNTY, CONNECTICUT

STREAM: OXOBOXO BROOK

DATE OF INSPECTION: 15 JUNE, 1978

### BRIEF ASSESSMENT

Wheeler Pon Dam is a masonry structure constructed in the early 1800's. The dam has a maximum height of 20.0 feet and is approximately 54.0 feet long. It is comprised of a 21.0 foot concrete ogee spillway at the left abutment and a 33.0 foot vertical masonry wall at the right abutment. The outlet works at the left abutment is a system of intake chambers and valve pits which withdraw a supply of process water for the adjacent mill complex. There are no plans, specifications, or computations available from the Owner, County, or State offices regarding the design, construction or repairs of this dam.

Due to its age, Wheeler Pond Dam was neither designed nor constructed by approved state of the art methods. Based upon the visual inspection at the site, the lack of engineering back-up data available, and no operational or maintenance evidence, there are areas of concern which must be corrected to assure the long term performance of this dam. The dam is considered to be in fair condition with the following visible signs of concern: the inoperable, low-level spillway gate, the poor condition of the mill intake and by-pass chamber and valve pit system now used for the control of water levels in the pond, the obstructed downstream channel and the apparent lack of maintenance of the dam and its appurtenances.

Hydraulic analyses indicate that the existing spillway can discharge a flow of 934 cubic feet per second (cfs) at Elevation (El.256.0) top of dam. A spillway design test flood of 6995 cfs (one half of the probable maximum flood) will overtop the dam by approximately 6.92 feet. In the improbable event of overtopping, complete failure of the dam could occur. Due to the potential for overtopping, it is recommended that a definite plan for surveillance and a warning system be developed for use during periods of unusually heavy rains and runoff.

It is recommended that the Owner engage the services of an engineer experienced in the design of dams to analyze the freeboard requirements with respect to the spillway design test flood and institute corrective measures to reduce the overtopping potential and improve the spillway capacity. Other action to be undertaken by the Owner should include the rehabilitation of the mill intake and by-pass chamber and valve pit system in order to control the water surface levels, restoration of the low level spillway gate to improve regulation of the pond levels, development of a regular program of maintenance and inspection, clearance of the dam site and downstream channel of debris and vegetal growth and preparation of an emergency action plan.

The above recommendations should be implemented within one to two years after receipt of the Phase I Inspection Report.

The alternatives to these recommendations would be draining the reservoir and maintaining the water surface at a reduced level.

C-E MAGUIRE, INC.

by

Richard W. Long, P.Z.

Vice President

This Phase I Inspection Report on Wheeler Pond Dam has been reviewed by the undersigned Review Board members. In our opinion, the reported findings, conclusions, and recommendations are consistent with the Recommended Guidelines for Safety Inspection of Dams, and with good engineering judgment and practice, and is hereby submitted for approval.

CHARLES G. TIERSCH, Chairman Chief, Foundation and Materials Branch Engineering Division

FRED J. RAVENS, Jr., Member Chief, Design Branch Engineering Design

**Engineering Division** 

SAUL COOPER, Member Chief, Water Control Branch **Engineering Division** 

APPROVAL RECOMMENDED:

ae B. Fregar JOE B. FRYAR

Chief, Engineering Division

SEP 11 8/3

### **PREFACE**

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation, and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued care and inspection can there be any chance that unsafe conditions be detected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway lest flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aide in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

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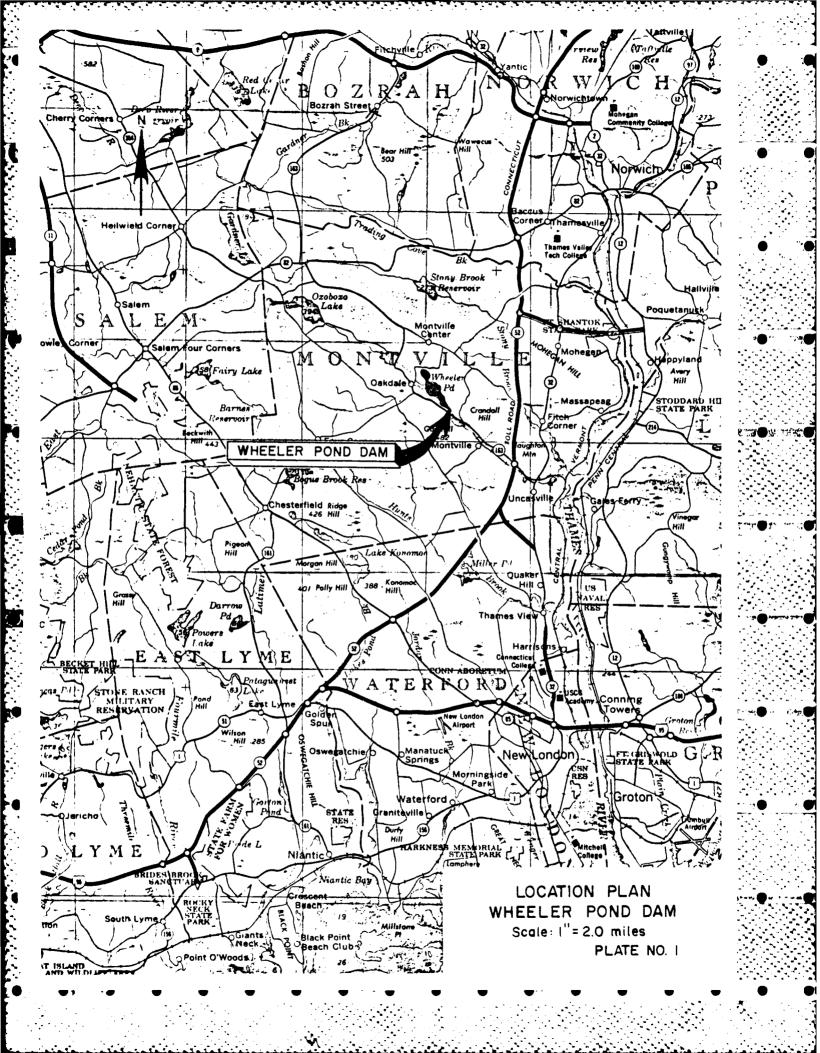
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C-I WHEELER POND DAM - LOOKING UPSTREAM



## NATIONAL DAM INSPECTION PROGRAM PHASE 1 INSPECTION REPORT

### WHEELER POND DAM

### SECTION 1

### PROJECT INFORMATION

### 1.1 GENERAL

Authority: Public Law 92-367, August 8, 1972, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a national program of dam inspection throughout the United States. The New England Division of the Corps of Engineers has been assigned the responsibility of supervising the inspection of dams within the New England Region. C-E Maguire, Inc. has been retained by the New England Division to inspect and report on selected dams in the State of Connecticut. Authorization and notice to proceed was issued to C-E Maguire, Inc. under a letter of 26 April, 1978 from Ralph T. Garver, Colonel, Corps of Engineers. Contract No. DACW33-78-C-0300 has been assigned by the Corps of Engineers for this work.

### b. Purpose:

- (1) Perform technical inspection and evaluation of non-Federal dams to identify conditions which threaten the public safety and thus permit correction in a timely manner by non-Federal interests.
- (2) Encourage and assist the States to initiate quickly effective dam safety programs for non-Federal dams.
- (3) To update, verify and complete the National Inventory of Dams.

### 1.2 DESCRIPTION OF PROJECT

a. Location: Wheeler Pond is located in the Oxoboxo Brook watershed of the Thames River Basin, approximately 1.0 mile northeast of Montville along Route 163, in New London County. Wheeler Pond has a surface area of 92.0 acres and drains a watershed of 9.09 sq. miles. The watershed is rural in character with rolling hills of woodland and swamps. The pond is oblong in shape and aligned in an approximate north-south axis with the dam located at the most southerly point.

### b. Description of Dam and Appurtenances:

wheeler Pond Dam is a masonry structure approximately 54.0 ± ft. long (including spillway) with a maximum height of about 20.0 ft. above streambed at the centerline of the dam. A concrete ogee spillway section 21.0 ft. long forms the left abutment of the crest section. The spillway crest elevation of the dam is 253.0 National Vertical Geodetic Datum (NGVD). The dam is located at a point where the valley is narrow and the bedrock forms steep abutments. The outlet works consists of a low level vertical slide gate located at the right abutment.

The dam provided process water for the Federal Paperboard Company located adjacent to the facility. Process water was withdrawn from the pond at the left abutment through an intake chamber and flowed through a 16" diameter steel pipe to the mill. Gated wet well chambers also located at the left abutment regulated the flow to the mill or the quantity to be by-passed and released downstream

- of the dam (See Sketch Appendix B). The dam is presently used to store supplemental process water supply for the Connecticut Paperboard Company located downstream at the confluence of Oxoboxo Brook and the Thames River.
- c. <u>Size Classification</u>: The dam is classified as intermediate in size because its impoundment capacity at spillway crest elevation is equal to 1000 Ac-Ft. which is the lower limit for that category under the Recommended Guidelines for Safety Inspection of Dams.
- d. <u>Hazard Classification</u>: The dam is a significant hazard potential category structure because it is located in a predominantly rural or agricultural area where failure may damage isolated homes, secondary highways or cause interruption of use or service of public utilities.
- e. Ownership: Records are not clear as to the early ownership of the dam, however, it is known that a Mr. T. Wisniewski, 996 Norwich Turnpike, Uncasville, 06382, its present owner,

purchased the dam in 1971 from the Federal Paperboard Company. The flowage and water rights are owned and controlled by the Connecticut Paperboard Company and used for process water supply at their downstream facility.

- f. Operator: Mr. L. Duchemin, Maintenance Supervisor
  Connecticut Paperboard Company
  Uncasville, Connecticut
  (203)-848-0681 home
  (203)-848-1500 business
- g. Purpose of the Dam: The dam is used to store
  a supplemental supply of process water for the
  Connecticut Paperboard Company located downstream primarily, with some limited recreational
  use.
- h. <u>Design and Construction History</u>: No data is available regarding design or construction.
- i. Normal Operating Procedures: Gates at Wheeler Pond Dam, normally, are not regulated during the year with the exception of the mill process water intake. This intake is opened in the spring slightly to provide a supply to the sprinkler system of the abandoned Federal Paperboard Company complex.

### 1.3 PERTINENT DATA

- Drainage Area: The Wheeler Pond Dam drainage basin located in New London County, Connecticut is generally triangular in shape oriented in a northwest-southeast axis. The basin is approximately 3.4 miles in length with an average width of 2.0 miles and a total drainage area of 9.09 sq. miles. The topography is generally rolling hills with swamp areas providing natural storage. Elevations range from a high of 600.0 NGVD to the spillway crest at 253.0 with basin slopes of a flat to moderate degree. Upstream natural and manmade storages include Oakdale, Schofield and Paris Pond and Oxoboxo Lake. Oxoboxo Lake Dam located in the upper reach of the basin controls 3.29 sq. miles of watershed. These storages tend to delay and dampen the peak runoff flowing to Wheeler Pond. The drainage basin is mainly undeveloped, wooded and agricultural in character. A general basin map is shown in Appendix D.
- b. <u>Discharge at Damsite</u>: The peak and shape of the inflow hydrograph is appreciably modified by

storages located upstream from Wheeler Pond Dam.

The adopted spillway design flood (1/2 PMF) is
equal to 830 csm or 7545 CFS and 6995 CFS as
inflow and outflow values respectively. (See
Appendix D) A flood of 100 year recurrence interval will be approximately 2200 CFS and 2126
CFS as inflow and outflow, respectively. No discharge records are maintained for this dam. Other
discharge values are listed below:

- 1. Outlet works (conduits) size 2'-6"x2'-6" and Invert El. 242.60.
- 2. Maximum known flood at damsite unknown.
- 3. Overflow spillway capacity at maximum pool elevation 934.0 CFS @ El. 256.0.
- 4. Gated outlet capacity at normal pool elevation 105.0 CFS @ El. 253.0 and tailwater El. 235.0.
- 5. Gated outlet capacity at maximum pool elevation 121 CFS @ El. 256.0.
- 6. Total discharge capacity at maximum pool elevation 1055 CFS @ El. 256.0.

### c. Elevations (ft. above MSL)

- 1. Top Dam 256.0

	3.	Full flood control pool	256.0 with no free- board
	4.	Recreation pool	253.0
	5.	Spillway crest	253.0
	6.	Upstream invert of Intake Structure	242.60
	7.	Streambed at center- line of dam	233.0 - 232.0
	8.	Maximum tailwater	Not computed
đ.	Rese	rvoir (Feet)	
	1.	Length of maximum pool	6150
	2.	Length of recreation pool	6150
	3.	Length of flood control pool	6150
e.	Stor	age (acre-feet)	
	1.	Recreation pool	1000
	2.	Flood control pool	276
	3.	Design surcharge	276
	4.	Top of dam	1276
	5.	Flood control pool of 270 0.57 inches of runoff fro of 9.09 sq. miles.	
f.	Rese	rvoir Surface (acres)	
	1.	Top dam	92.0
	2.	Maximum pool	92.0

	3.	Flood-control pool	92.0
	4.	Recreation pool	92.0
	5.	Spillway crest	92.0
	6.	One foot of surcharge re of runoff from its drain square miles.	
g.	<u>Dam</u>		
	1.	Type	Stone masonry and concrete
	2.	Length	33.0 ft. stone ma- sonry + 21.0 ft. concrete
	3.	Height	20.0
	4.	Top Width	5.0
	5.	Side Slopes	Vertical Face Ma- sonry
	6.	Zoning	N/A
	7.	Impervious Core	N/A
	8.	Cutoff	N/A
	9.	Grout Curtain	N/A
	10.	Other	N/A
h.	Spil	lway	
	1.	Type	Overflow uncon- trolled
	2.	Length of Weir	Total length 54.0 21.0 ogee type + 33.0 free overfall type

- 3. Crest elevation 253.0
- 4. Gates Vertical Slide Gate (Inoperable)
- 5. U/S Channel Curved Natural Bed
- 6. D/S Channel Stony Natural Bed and Bed Rock
- 7. General N/A

### i. Regulating Outlets

- 1. Invert 242.6
- 2. Size 2.6'x2.6'
- 3. Description Manually operated

vertical

- 4. Control Mechanism Slide Gate located on spillway of dam. Inoperable on the day of inspection.
- 5. Other ---

### SECTION 2

### ENGINEERING DATA

### 2.1 DESIGN

No engineering data for this dam is available.

### 2.2 CONSTRUCTION

No record of the construction or repairs, if any, exist.

### 2.3 OPERATION

No records of the operation of this facility have been maintained.

### 2.4 EVALUATION

- a. Availability: No specific information is available to permit evaluation of design parameters and construction practices employed.
- b. <u>Adequacy</u>: Available data is inadequate. Design parameters must be assumed.
- c. <u>Validity</u>: Validity of limited data must be verified.

### SECTION 3

### VISUAL INSPECTION

### 3.1 FINDINGS

a. General: The dam is utilized only as an auxilliary supply of process water for downstream
use and therefore not being actively used appears
to be neglected and poorly maintained. Brush and
trees overgrow the structure, gates are leaking

and inoperable and valve chambers are open, filled with debris and subject to vandalism. Access to the dam and its appurtenances is not restricted and hence the damsite suffers from misuse. The intake chamber and by-pass valve pit system is filled with debris and the valves are leaking. The appearance of the dam is poor and its condition is deteriorating.

which appears to have had two channels. The deeper channel is now occupied by the dam.

The dam appears to be founded on bedrock, and both abutments are quartzite. The bedrock bedding strikes parallel to the dam crest and dips about 45° toward the reservoir. One joint set strikes about N60°E and dips 40° to 70°s. A conjugate joint set strikes about N15°W and dips 57° to 84°W. All bedrock jointing appears to be tight with no leaks observed.

The downstream face of the dam appears to have

The downstream face of the dam appears to have significant seepage exiting through the joints in the dry masonry face. Other joints appear open but are not leaking.

The joints in the bedrock abutments are tight and show no signs of seepage downstream of the dam. However, one block of bedrock in the right abutment just downstream of the dam appears to be loosening.

tion of the dam appeared to be inadequate in size.

Freeboard for the structure, as observed, was

minimal.

The low level sluice gate (outlet works) in the vertical masonry section of the dam was closed and inoperable. The access bridge to the gate control was badly weathered and in disrepair. The right abutment training wall has settled with stones dislodged and loosened.

The mill process water intake and by-pass system is severely neglected. It consists of uncovered pits with leaking valves, valve chambers filled with debris and rubbish, stop logs and trash racks rotted or broken and the adjacent terrain overgrown with brush and trees. Continued deterioration and neglect of this mill intake and by-pass system will lead to loss of regulation of the reservoir levels with subsequent detrimental effects on the dam.

- d. Reservoir Area: Generally, the pond shoreline is heavily wooded with moderately to steep sided terrain. The heavy growth should preclude the occurrence of slides or sloughs and subsequent sedimentation. However, this heavy growth adjacent to the dam and mill intakes could cause clogging if not monitored periodically.

  The intake and approach channel to the mill was filled with leaves and debris and seriously restricted in operation.
- e. <u>Downstream Channel</u>: The downstream channel is naturally winding and confined but is now additionally restricted with debris and vegetative overgrowth. The 12.0 ft. high stone wall on the right side of the downstream channel has collapsed over a length of 6.0 ft. A 4 to 5 inch diameter tree is now growing in the zone from which the stones collapsed.

The spillway channel is a natural bedrock streambed. Trees up to 5.0 inches in diameter overhang the channel.

### 3.2 Evaluation

- the dam does not appear to adversly effect the stability of the dam.
- b. The potential exists for an additional collapse of the wall on the right side of the downstream channel. A further collapse of this wall could severely effect the foundations of the adjacent mill.
- c. The non-functioning of the low level sluice gate at the right abutment directly effects the drawdown capabilities of the dam's operation.
- d. The poor, deteriorated condition of the mill intake and by-pass system could lead to its malfunction and hence loss of control of water surface levels at the dam.

### SECTION 4

### OPERATIONAL PROCEDURES

### 4.1 Procedures

Wheeler Pond Dam water surface levels are not regulated. The storage is maintained as a supplemental supply for downstream use. Gates remain closed and generally

unused. In the spring, the intake to the abandoned Federal Paperboard Company complex adjacent to the dam, is opened slightly to provide a supply of water to the sprinkler system. No formal operational procedures are followed for this facility.

### 4.2 Maintenance of Dam

The dam is not maintained. Its condition warrants an active program of rehabilitation to insure its continued service.

### 4.3 Maintenance of Operating Facilities

Operating facilities at the dam are neglected with gates and valves inoperable or partially impaired in use. Valve pits are open and filled with trash and debris and in generally poor condition. Lack of maintenance of these facilities is apparent.

### 4.4 Description of Any Warning System in Effect

No formal warning system is used. Personnel at the Connecticut Paperboard Company who control the water rights at the dam, monitor local weather forecasts for the approach of high intensity storms. No inspection or monitoring program for the dam is in effect or emergency action plan to reduce or minimize the effects of downstream damages in the event of an emergency situation.

### 4.5 Evaluation

Operational and maintenance procedures for this dam have not been developed or followed. In view of the neglected condition of this dam and its appurtenances, it is important that the Owner institute a monitoring and a regular inspection program as soon as practicable.

An operational procedure to follow in the event of an emergency should also be adopted.

### SECTION 5

### HYDRAULIC/HYDROLOGY

### 5.1 Evaluation of Features

a. Design Data: No specific design data is available. In lieu of existing information, U.S.G.S. topographic mapping was used to develop several hydraulic and hydrologic parameters. Storage at crest elevation in the pond was estimated in the absence of elevation-storage graphs. Outflow for the Spillway Test Flood inflow was developed according to Corps of Engineers guidelines assuming the pond level at spillway crest (See Appendix D). Some pertinent hydraulic design data was obtained for the spillway and outlet works by field measure-

# INFLOW, OUTFLOW AND SURCHARGE DATA

FREQUENCY IN YEARS	24-HOUR TOTAL RAINFALL IN INCHES	24-HOUR* EFFEC- TIVE RAINFALL IN INCHES	MAXIMUM INFLOW IN C.F.S.	MAXIMUM** OUTFLOW IN C.F.S.	SURCHARGE STORAGE IN FEET	SURCHARGE STORAGE ELEVATION
10	5.0	2.6	1240	1160	3.80	256.80
20	6.5	4.1	1960	1870	5.20	258.20
100	7.0	4.6	2200	2126	5.34	258.34
1/2 MPF	11.9	9.5	7545	6995	9.92	26. 696
TEST FLOOD	21.4	19.0				
- M.P.F.						

 $^{1.}$   $^2_{10},^2_{50},^2_{100},$  inflow discharges computed by approximate methodology of Soil Conservation Service.

at spillway crest elevation 253.0

\*Infiltration assumed as 0.1"/hour

initially full

of dam =

(Top

\*\*Lake assumed

- 1/2 MPF and "test flood" computation based on COE instructions and guidelines
  - 3. Maximum capacity of spillway without overtopping the top of the dam elevation C.F.S. is equal to 934
- All discharges indicated are dependent upon the continued integrity of upstream storage reservoirs.
- Surcharge storage is allowed to overtop the dam when exceeding the spillway capacity.

NOTES:

ment during the visual inspection. Due to lack of downstream data, the dam failure profile was not calculated but an approximation was made of the depth of normal flow due to the failure of the dam. Surcharge storage and overtopping were calculated assuming that the Wheeler Pond surface area remained constant above the spillway crest elevation.

- b. <u>Experience Data</u>: No historial data for discharges or water levels have been recorded for Wheeler Pond Dam.
- c. <u>Visual Observations</u>: The following detrimental items were observed or calculated requiring analysis and correction.
  - 1. The freeboard allowance is inadequate.
  - 2. The natural downstream channel is overgrown and obstructed with loose rock and debris.
  - 3. The low level spillway gate is inoperable.
  - 4. The access bridge to the spillway gate is in serious disrepair. It should also be noted that the spillway gate bridge will act as a debris collector and should be periodically inspected.
  - 5. The process water system of intake and valve chambers needs cleaning and rehabilitation.

- 6. Vegetation must be cleared and maintained from the dam and adjacent areas.
- Overtopping Potential: Wheeler Pond Dam will not pass the recommended spillway design flood (1/2 PMF) without overtopping the dam, and therefore the spillway capacity is inadequate. The maximum spillway discharge capacity is equal to 934 c.f.s. without overtopping which represents 14% of the "Test Flood" outflow discharge of 6995 The spillway capacity is judged as serious, since inadequate freeboard for even lesser storm events will cause overtopping of the structure. Wheeler Pond is generally oblong in shape with its axis oriented in a WNW-ESE direction. With a fetch of approximately 6000.0 ft. and a converging channel approach to the dam, wind generated wave surge and ride up have a distinct potential to overtop the structure.

It is estimated that a failure of this dam would cause a failure discharge that would overflow route 163 downstream by approximately 4.0 feet and that water levels in the pond downstream of the highway will, correspondingly, be 3.0 to 4.0 feet higher. Location of this large pond downstream should

quickly dissipate the unsteady flow energy and result in flooding only.

### SECTION 6

### STRUCTURAL STABILITY

### 6.1 Evaluation of Structural Stability

- a. <u>Visual Observations</u>: There were no visual signs of structural instability or distress in the dam at the time of the inspection.
- b. <u>Design and Construction Data</u>: No data is available on which to base an evaluation.
- c. Operating Records: There are no records of operation available.
- d. <u>Post Construction Changes</u>: No data is available regarding construction changes.
- e. <u>Seismic Stability</u>: This dam is in seismic zone 1 and hence, does not have to be evaluated for seismic stability, according to the recommended guidelines.

### SECTION 7

### ASSESSMENT, RECOMMENDATIONS AND REMEDIAL MEASURES

### 7.1 Dam Assessment

- a. <u>Condition</u>: Based on examination of available documents and visual inspection of the Wheeler Pond Dam and its appurtenant structures, the dam is judged to be in fair condition. However, there are areas of concern which must be corrected to assure the long term usefulness of this facility.
  - 1. The inoperable low level spillway gate must be restored to service to increase the capacity of the present outlet system and its access bridge upgraded.
  - 2. A detailed examination should be made of the wall on the right side of the spillway as it appears to be potentially unstable and could lead to failure of the foundations of the adjacent mill building and clogging of the downstream channel.
  - 3. The spillway capacity does not satisfy the screening criteria established in the recommended guidelines for the spillway design test flood.
  - 4. Present marginal freeboard allowances can potentially permit overtopping of the structure under moderate storm events and wind conditions.
  - 5. The poor condition of the mill complex intake and by-pass system of chambers and valve pits must be rehabilitated immediately since it is the sole means at present to regulate water levels in Wheeler Pond.

- 6. Potential clogging of the bridge structure at route 163 exists due to the debris and vegetal growth of the downstream channel.
- b. Adequacy of Information: The information available is such that the assessment of the condition of the dam must be based primarily on the visual inspection and the past operational performance of the structure.
- c. <u>Urgency</u>: The recommendations and remedial measures outlined below should be implemented within a one to two year period.
- d. <u>Necessity for Additional Investigations</u>: Investigations to further assess the adequacy of the dam and its appurtenances are necessary. Sufficient engineering data must be obtained to implement the recommendations listed below.

### 7.2 Recommendations

- a. <u>Facilities</u>: In view of the concerns for the long term condition of Wheeler Pond Dam, and the lack of engineering back-up data, it is recommended that the following measures be undertaken by the Owner:
  - 1. That the Owner obtain the services of an engineer experienced in the design of dams to analyze the freeboard allowance with respect to the Spillway Design Test Flood

criteria and institute corrective measures to reduce the overtopping potential and improve the spillway capacity.

 Examine and rehabilitate the existing masonry wall at the right abutment of the spillway to prevent damage or collapse of the adjacent mill structure foundation and clogging of the downstream channel of the dam.

### 7.3 Remedial Measures

- 1. Alternatives: Alternatives to the recommendations listed above would be to lower the water levels in Wheeler Pond at the approach of high intensity storms or expected rainfall periods to increase the impoundments' capacity for flood control or to consider increasing upstream storage capacity at other facilities to relieve the conditions at Wheeler Pond Dam.
- 2. Operations and Maintenance Procedures: While the dam is judged to be in fair condition, it is considered extremely important that the following items be attended to as soon as possible:
  - a. Immediately institute a program to clear and rehabilitate the intake chamber and valve pit system leading to the abandoned mill complex, in order to maintain control of the water levels at Wheeler Pond.
  - b. Drawdown the water level in order that the low level gate at the spillway can be repaired or replaced which will increase the outlet capacity for emergency and repair situations.

- c. Remove and dispose the debris and vegetal growth from the dam and its appurtenant structures and the downsteam channel.
- d. Develop and implement a regular program of monitoring, inspection and maintenance of the facility.
- e. Prepare an emergency action plan to prevent or minimize the impact of dam failure, listing the expedient action to be taken and authorities to be contacted.

# APPENDIX A INSPECTION CHECK LIST

## VISUAL INSPECTION CHECK LIST PARTY ORGANIZATION

PROJECT Wheeler Pond Dam	DATE 14 June 1978
	TIME 0930-1500
	WEATHER Clear
	W.S.ELEVU.SD. 9
PARTY:	
R. Long - C-E Maquire*	6. <u>I. Wisniewski - Owner</u>
2. <u>S. Khanna - C-E Maguire</u>	7. H. Beetham - 1st Selectmen
3. Poulos - GEI	8. W. Staubley - Gate Tender
R. Brown - C-E Maguire	J. Rodgers - Civil Defence
K. Dalenberg - GEI 5.	
PROJECT FEATURE	INSPECTED BY REMARKS
1.	•
2	
3	
4	<del></del>
5	
6	
7	
8	
9	
0	

### PERIODIC INSPECTION CHECK LIST PROJECT Wheeler Pond Dam DATE <u> 14 June 1978</u> DISCIPLINE INSPECTOR \_\_\_\_\_ DISCIPLINE INSPECTOR CONDITION AREA EVALUATED DAM Crest Elevation 253 NGVD Current Pool Elevation 253 NGVD Maximum Impoundment to Date Surface Cracks Not Applicable Pavement Condition Not Applicable Movement or Settlement of Crest Not Observable Lateral Movement None Observed Vertical Alignment No misalignment apparent Horizontal Alignment Condition at Abutment and at Concrete Condition appears good except for loose Structures block of bedrock noted below under sloughing. Indications of Movement of Structural None observed Items on Slopes Trespassing on Slopes Free access, no difficulties observed. Sloughing or Erosion of Slopes or A block of bedrock several feet in Abutments dimension is loose. Block is in right abutment down stream of dam. Rock Slope Protection - Riprap None present Failures Unusual Movement or Cracking at on None observed near Toes Unusual Embankment or Downstream None observed Seepage Piping or Boils None observed Foundation Drainage Features None apparent Toe Drains None Instrumentation System None

PERIODIC INSPECT	TION CHECK LIST	
PROJECT Wheeler Pond Dam	DATE 14 June 1978	
INSPECTOR	DISCIPLINE	
INSPECTOR	DISCIPLINE	
AREA EVALUATED	CONDITION	
DAM (Cont.)		•
Vegetation	None on dam, trees are growing from bedrock abutments. Downstream completely overgrown.	
		Sen O Y 11 And O

PERIODIC IN	SPECTION CHECK LIST	ja j
PROJECT Wheeler Pond Dam	DATE <u>14 June 1978</u>	
INSPECTOR	DISCIPLINE	
INSPECTOR	DISCIPLINE	
AREA EVALUATED	CONDITION	
DIKE EMBANKMENT	NONE	
		man market m
		Em⊕y scried
		<u>*</u> ~●
		Series of the series of

PERIODIC INSPECT	ION CHECK LIST	
PROJECT Wheeler Pond Dam	DATE	
INSPECTOR	DISCIPLINE	
INSPECTOR	DISCIPLINE	
AREA EVALUATED	CONDITION	
OUTLET WORKS - INTAKE CHANNEL AND INTAKE STRUCTURE		
a. Approach Channel	Natural bed	
Slope Conditions	Gradual	*********
Bottom Conditions	Natural bed with weed growth	
Rock Slides or Falls	Yes - along right shoreline above gate.	
Log Boom	None	e sale y mare
Debris	Yes, trees, refuse	
Condition of Concrete Lining	Not applicable	
Drains or Weep Holes	Not observed	have One to the
b. Intake Structure		
Condition of Concrete	Net postinali	
Stop Logs and Slots	Not applicable	A STATE OF THE STATE OF
· ·		
• •		
·.		
1		

PERIODIC INSPECT	TION CHECK LIST	•
PROJECT Wheeler Pond Dam	DATE 14 June 1978	
INSPECTOR	DISCIPLINE	
	DISCIPLINE	
AREA EVALUATED	CONDITION	
OUTLET WORKS - Gate structure on Spillway. See note below. a. Structural		
General Condition	Poor	•
Condition of Joints	Open stone masonry	
Spalling	None observed	
Visible Reinforcing	Not applicable	857 <b>3 4</b>
Rusting or Staining of Concrete	Not applicable	
Any Seepage or Efflorescence	Yes, through masonry joints	
Joint Alignment	Fair	i-e-on
Unusual Seepage or Leaks in Gate Chamber	Gate not seated - unable to determine extent of leakage.	
Cracks	Cannot be observed	
Rusting or Corrosion of Steel	Considerable where visible above water line.	
b. Mechanical and Electrical	Not applicable	
	NOTE: Gate mechanism consists of vertical lift slide gate operated by 6" x 6" timber post with rack and pinion gearing. Gate is inoperable.	
•		
X		

PERIODIC INSPEC	CTION CHECK LIST
NECT Wheeler Pond Dam	DATE <u>14 June 1978</u>
PECTOR	DISCIPLINE
PECTOR	DISCIPLINE
AREA EVALUATED	CONDITION
TLET WORKS - By-pass gate chambers	
Concrete and Structural	
General Condition	Poor, not maintained, open pits partially filled with debris, rusted equipment. See note below.
Condition of Joints	Good
Spalling	None observed
Visible Reinforcing	None observed
Rusting or Staining of Concrete	None observed
Any Seepage or Efflorescence	None observed
Joint Alignment	Not applicable
Unusual Seepage or Leaks in Gate Chamber	None observed
Cracks	None observed
Rusting or Corrosion of Steel	Considerable
Mechanical and Electrical	None
	NOTE:
	Gates, stems, cleanout, trash rack and stop logs in poor condition. Bonnet on gate valve to plant cracked and leaking. Intake filled with debris, stop logs rotten, trash rack useless. Cleanout chamber filled with debris, cleanout burried. All chambers consist of open pits with no covering.

PERIODIC INSPECT	TION CHECK LIST	
PROJECT Wheeler Pond Dam	DATE 14 June 1978	
INSPECTOR		
INSPECTOR		
MOFECTOR	DISCIPLINE	
AREA EVALUATED	CONDITION	
OUTLET WORKS - TRANSITION AND CONDUIT	Not applicable.	
		Ann der sin der sein
		R. THE LANGE
·		
	•	
		A STANFORM
		h and walk
		in season of the service of the
<u>.</u> 3		
A-8		

OJECT Wheeler Pond Dam	DATE 14 June 1978
SPECTOR	DISCIPLINE
ISPECTOR	DISCIPLINE
AREA EVALUATED	CONDITION
OUTLET WORKS - SPILLWAY WEIR, APPROACH AND DISCHARGE CHANNELS	
a. Approach Channel	
General Condition	Curved with natural bed underwater, one or two feet deep on left side, deepens at outlet gate.
Loose Rock Overhanding Channel	None observed.
Trees Overhanging Channel Floor of Approach Channel	Yes, trees up to 4 inch diameter overhang channel. Natural rocky bed with weed growth.
b. Weir	Stone masonry with concrete cap. Spillway is divided into two sections
General Condition of Concrete	<ol> <li>Concrete ogee overflow.</li> <li>Vertical stone masonry overflow.         Some wear noted.</li> <li>Concrete portion - good condition,</li> <li>Stone portion - fair condition.</li> </ol>
Rust or Staining	None observed.
Spalling	None observed.
Any Visible Reinforcing	None observed.
Any Seepage or Efflorescence	Seepage through open joints of stone masonry observed.
Drain Holes	None.
. Discharge Channel	
General Condition	Natural bedrock - irregular
Loose Rock Overhanding Channel	Yes - See note "A".
Trees Overhanging Channel	Yes - Up to 5 inch diameter.
Floor of Channel	Natural bedrock.
Other Obstructions	Debris

	PERIODIC INSPECT	
PROJECT	Wheeler Pond Dam	DATE 14 June 1978
INSPECTOR		DISCIPLINE
INSPECTOR		DISCIPLINE
	AREA EVALUATED	CONDITION
"A" Stone Colla	e wall on right side of channel apsed portion about 12 ft. high inch diameter tree now growing	has caved into channel. and 6 ft. wide. Four to in opening.

	PERIODIC INS	PECTI	TON CHECK LIST
PROJECT	Wheeler Pond Dam		DATE 14 June 1978
INSPECTOR			DISCIPLINE
INSPECTOR			DISCIPLINE
	AREA EVALUATED		CONDITION
OUTLET WO	RKS - SERVICE BRIDGE		
Bear Anch Brid Long Unde Seco Deck Drain	or Bolts  ge Seat  itudinal Members  r Side of Deck  ndary Bracing  nage System  ings  nsion Joints		Wood access bridge in poor condition. Exposed to severe ice damage heave, etc.
b. Abutme Gener Align Appro	ent & Piers ral Condition of Concrete nment of Abutment pach to Bridge ition of Seat & Backwall		None

### APPENDIX B

- 1. Listing of Locations for Available Correspondence
- 2. Copies of Past Inspection Reports
- 3. Plans, Sections, Details

### APPENDIX B-1

No design, construction or maintenance records could be located.

### APPENDIX B-2

Water Resources Commission - Supervision of Dams Inventory Sheet dated 25 November, 1964.

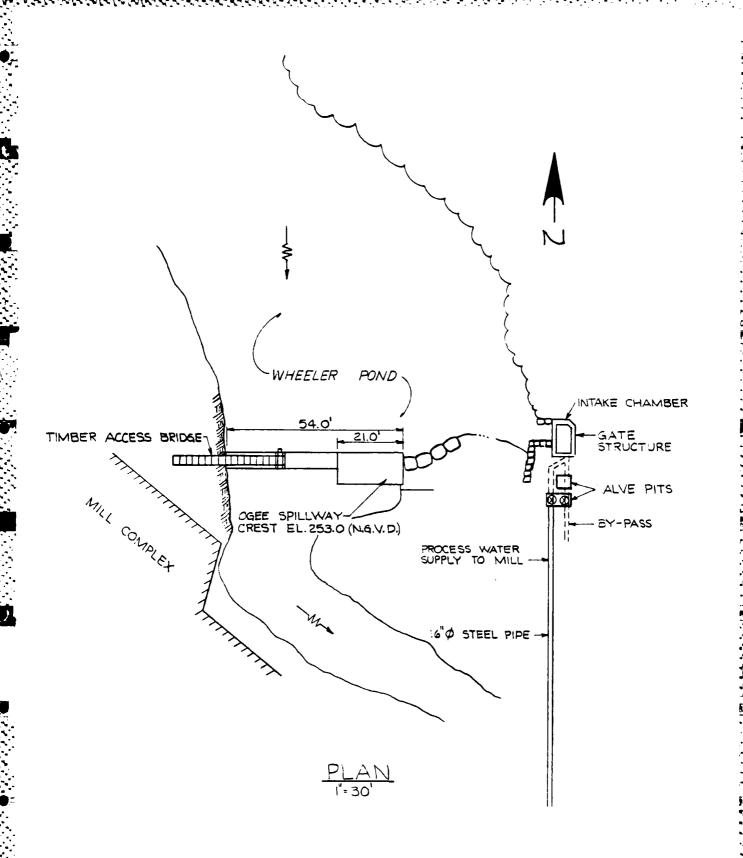
MV-30	WATER RESOURCES COMMISSION	, , , , , , , , , , , , , , , , , , ,	•
toried LV/S	SUPERVISION OF DAMS INVENTORY DATA	27 239	
LVPS	Invaliant billi	·	
25 NAVEMBER 1964	-		
Name of Dan or Pond	WHEELER POND		•
Code No	9.4 0×2.9		
Nearest Street Locat	ion ROUTE 163		
Town	P(U): (1): 1 =	nng 72-08.5	•
U.S.G.S. Quad	MONTVILLE	-A-1 41-27,9 th 7/7:	
Name of Stream	oxoboxo Brook		ا حالم
Owner FEDERAL	PANER BUARD CO. INC.	Theritare Wisniew?	** <b>●</b> **
Address	ALONTYIERE	Box-326.	
		`	<b>以●</b> 花/4000000000000000000000000000000000000
Pond Used For 12	DUSTILLAL WATERS SUPPL	У	
	Width 800 FEET Length 500	7/· 8	
Total Length of Dam	Length of S	pillway 25 FEET	
Location of Spillway	ENTIRE DAM	,	
Height of Pond Above	Stream Bed 27 Fart		
Height of Embankment	Above Spillway 3 FEET		
Type of Spillway Cor	istruction CONCRETE, A	MISONBY	
Type of Dike Constru	ection CONCRETE 414	sonily - ledon	
Downstream Condition	is culvert under R	CUTE IGS AND	
	ROCKLAND POND		
Summary of File Data	1		
Remarks Scient	LEAKAGE NOTED IN	MASONRY	
PORTION	OF DAM		<b>.</b> • • • • •
	Damage? YES	Class B	

gade is reaking a new reaks arranged wall are made a second and and a second and and a second a second and a second and a second and a second and a second a

approve of 5 Wilson

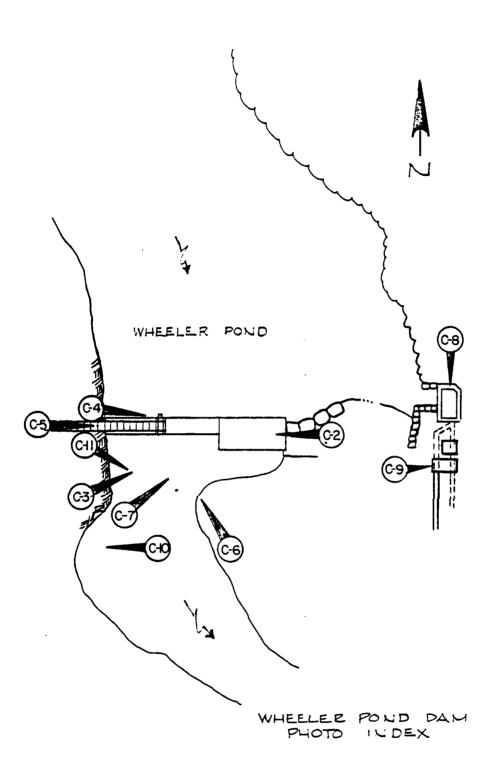
10-7-77 - 2" WATER OURA STWY MADE IT DIFFICULT TO NO LEMES MASSMRY APPRAISE SOUND. SOME PRINT OTHERS.

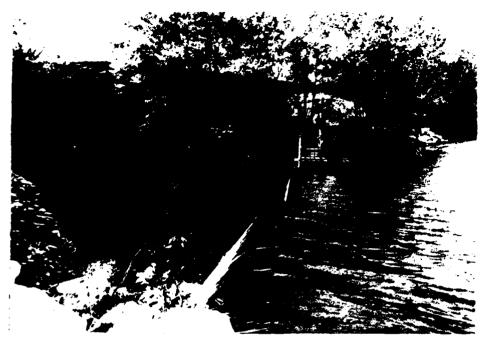




WHEELER POND DAM

# APPENDIX C SELECTED PHOTOS





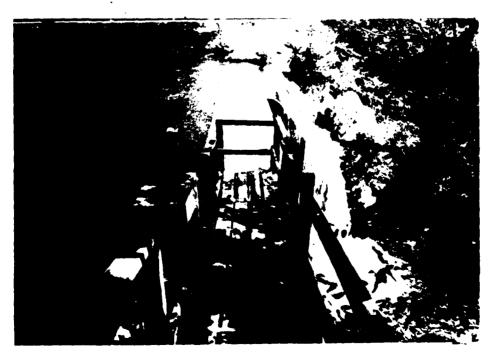
C-2 SPILLWAY CREST-LOOKING TOWARD RIGHT ABUTMENT (NOTE: CLOSE PROXIMITY OF MILL BUILDING.)



C-3 SPILLWAY CREST - LOOKING TOWARD LEFT ABUTMENT



C-4 OPERATING MECHANISM FOR LOW LEVEL SPILLWAY GATE OUTLET.



C-5 SERVICE BRIDGE LEADING TO OUTLET CONTROL



C-6 DOWNSTREAM FACE OF DAM. (NOTE: DARK AREA BELOW SERVICE BRIDGE - OUTLET GATE OPENING.)



C-7 DOWNSTREAM FACE OF DAM LOOKING TOWARD LEFT ABUTMENT



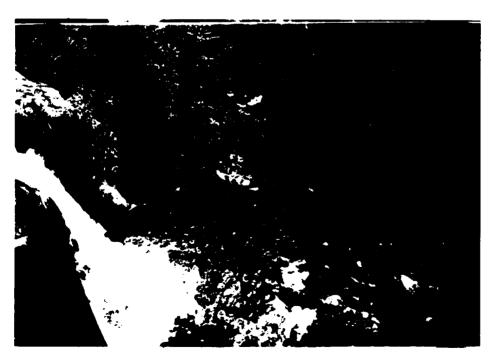
C-8 UNCOVERED INLET CHAMBER FOR BY-PASS. (NOTE: DEBRIS AND DETERIORATED STOP LOGS IN BACKGROUND.)

C-9 VALVE PIT-FOR PROCESS WATER SUPPLY TO MILL AND BY-PASS



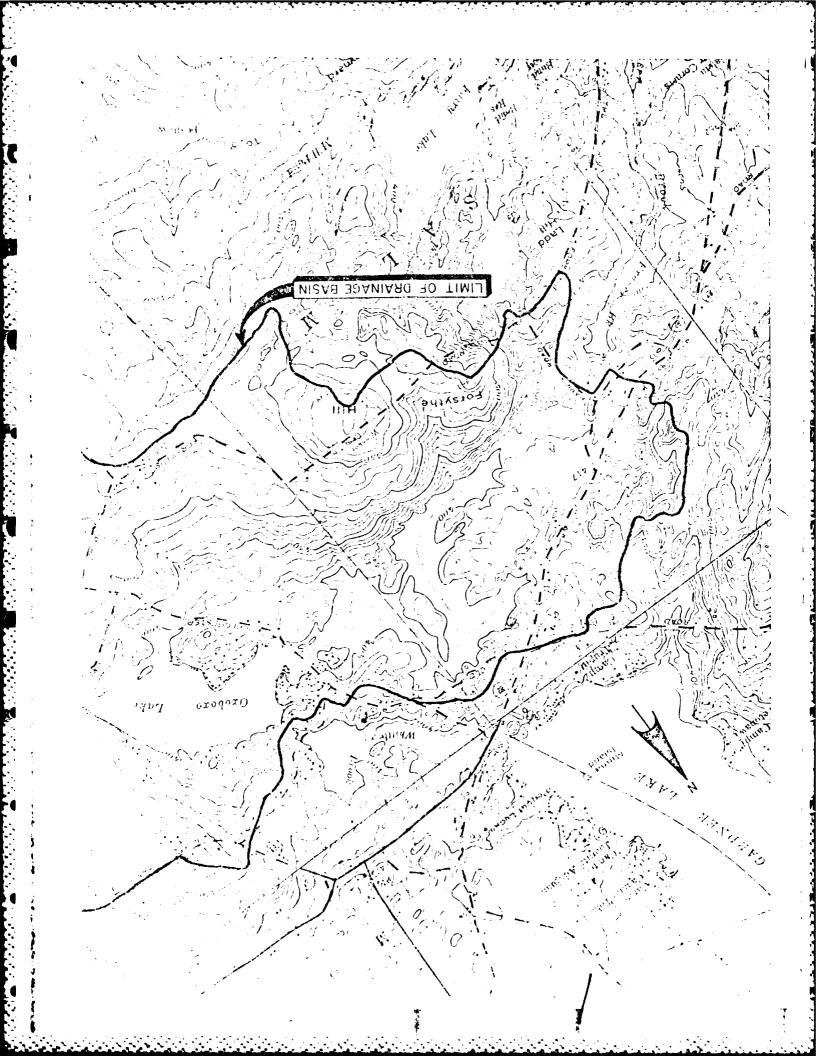


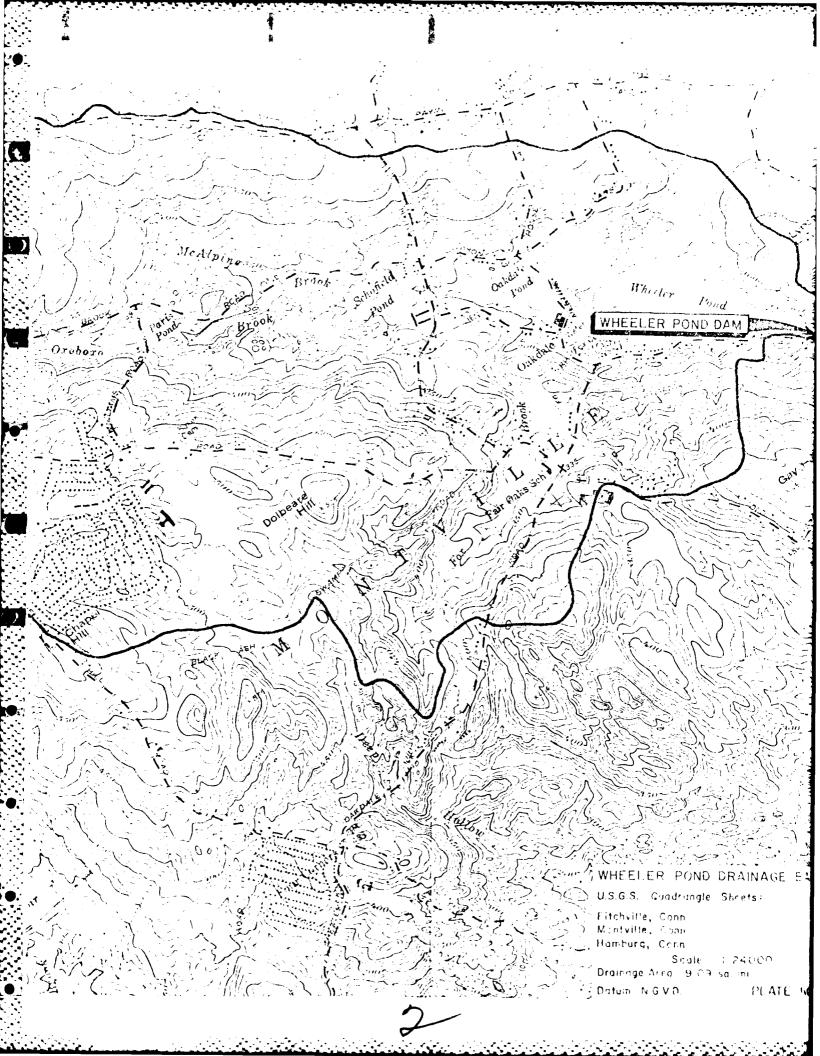
C-IO WALL AT RIGHT ABUTMENT OF DAM



C-II DOWNSTREAM CHANNEL

# APPENDIX D HYDROLOGIC COMPUTATIONS





0110 0140011 10401011		
Height of Dam =	<u>21.0</u> feet;	Hence <u>SMALL</u>
at prest elevation re	eservoir storage =	AC-ft., hence INTERMEDIATE
adopted size ca	ategory <u>INTERMEDIAT</u>	<u>E</u>
B. <u>Hazard Potential</u>		
DAM IS LOCATED	IN A PREDOMINANTLY	RURAL AREA AND ITS FAILURE
WILL DAMAGE O	R BREACH RT. 163 A	ND MAY CAUSE A LOSS OF A
IEW LIVES AND	HOMES LOCATED	DOWNSTREAM. THERE WILL BE
APPRECIABLE ECC	dnomic loss due t	O FLOODING PROBLEM. IT IS
A SIGNIFICANT	HAZARD PROGRAM	<u> </u>
It is estimated from	the rule of "thumb" fai	lure hydrograph as follows: ,
Category	Loss of Life	Economic Loss
		Homes = YES
SIGNIFICANT	YES	Buildings = YES
<del></del>		Farms = YES
		Miscellaneous = YES
		Highways or roads = YES
7. Hadard	<u>Size</u>	"Test Flood" on Spillway Design Floot
SIGNIFICANT	INTERMEDIATE	1/2 PMF TO PMF
Adopted S.S.F. test (1991)	= V2 PAGE	
		222
stantet value ni fes	t flood due to watershed	characteristics = <u>830</u> CSM

Date of Inspection:

Estimating Maximum Probable Discharges - Inflow and Outflow Values

loo feet	Outflow Characteristics Outflow Characteristics Outflow Characteristics First Approximation Second Approximation	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	7 8 9 10 11 12 13 1 <sup>1</sup> 4	30 2,96 1.81 9,54 6515 1.88 9.92 6995
i i	acteris ximatic			
	Char	h th	10	<u><u><u>a</u></u></u>
ĵ	Outflow Second		6	<u>.</u>
1 1	istics	S <sub>2</sub>	8	2.96
001	Character: proximation	h in feet	7	10.30
 	Outflow First Ap	4p crs	9	7545
f Dam=	istics	S QP	5	96'2
Length of Dam=	Inflow Characteristics	h in feet	77	10.30
	Flood	CFS	~	7545
	Test Flood		~	0
	Mame	u u	_	WHEELER POND

p = Discharge; h = suregarge height S = Storage in inches

### Tremmanmy ing Gomential

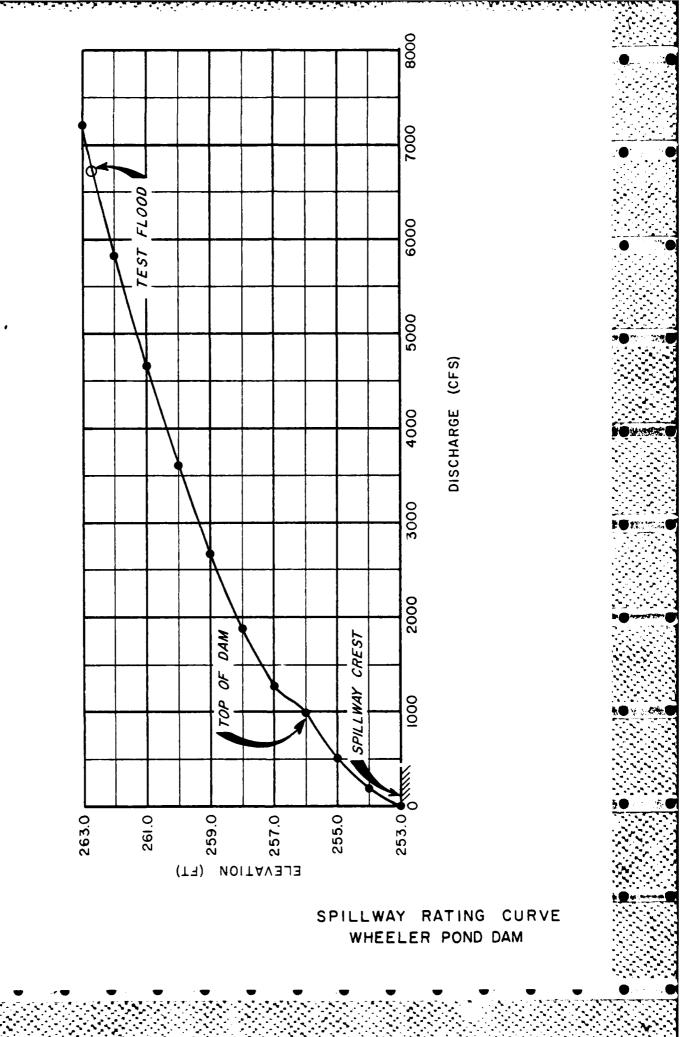
Spillway prest elevation =	253.00	M.S.L.
Top of dam elevation =	256.00	M.S.L.
Maximum discharge capacity of Spillway without overtopping	934	C.F.S.
"Test flood" outflow discharge =	6995	C.F.S.
<pre>f of "Test flood" carried by ) Spillway without overtopping ) =</pre>	13.3 %	1
"Test flood" outflow discharge = which flows over the dam	6061	C.F.S.
=	<b>86.7</b> % of "Te	st flood" 2

1 + 2 = 100%

### "Rule of Thumb Guidance for Estimating Downstream Dam Failure Hydrograph"

### BASIC DATA

Name of dam NASSLEA FONG DAM	Name of town MONTVILLE, CT -
Drainage area = 9.09 59.41	Top of dam <u>256.0</u>
Spillway type = <u>OGEE AND BROOD CRESTED</u>	Crest of spillway 253.0
Surface area at crest elevation = 92.	, and the second of the second
Reservoir bottom near dam = <u>242.60</u>	•
Assumed side slopes of embankments =	/:2
Depth of reservoir at dam site /7.0	. y= /7.0 FT.
Mid-height elevation of dam =	
Length of dam at crest = 54.0 Fr.	
Length of dam at mid-height = $44.0 F$	7
40% of dam length at mid-height = W <sub>b</sub> =	
Stream height of dam = 2/.0	FT.
Hydraulic height of dam = 17.0	F7
Step 1:  Rescive 5	
Elevation Estimated St M.S.L. In AC-ft.	
;	
- 92.0 Ac-t	7
•	
Step 2:	en comme de las americas ambien de las de las ambiens en las alles de las alles de las de las de las de las de Las desembles de las americas de las de las desembles de las dellas de las dellas de las dellas dellas dellas d Las dellas
$Q_{\rm pl} = \frac{8}{27} W_{\rm b} \sqrt{g} y_{\rm o}^{3/2}$	
	$v^{3/2} = 2072 CFS$



12 (

# APPENDIX E INFORMATION AS CONTAINED IN THE NATIONAL INVENTORY OF DAMS

# 開発器 INVENTORY OF DAMS IN THE UNITED STATES

PURPOSES TREAM TREAM TO CONSTR	(a)	NAME LATTI USE LUNGITUDE MEGATI LATE  ONORTH) (WEST) DAY MO YN	POND DAM	THAME OF MADUNOMENT	AMERIEM POND		NEAREST DOWNSTREAM FROM DAY CITY-TOWN-VILLAGE (MI.)	MONTVILLE 15000	8 (9) (9) (9) (9) (9) (9) (9) (9) (9) (9)	TATAL MONING ACTION OLD CAR		REMARKS		1 1	NAVIGATION LOCKS		(e)	ENGINEERING BY CONSTRUCTION BY		(a)	CONSTRUCTION OPERATION MAINTENANCE	CONN PAPERBUARD CO STATE UP CT		DAY MO YR AUTHORITY FOR INSPECTION	14JUN78 PL 92=367	
--------------------------------	-----	--	----------	---------------------	--------------	--	---	-----------------	---	-----------------------------	--	---------	--	-----	------------------	--	-----	--------------------------------	--	-----	------------------------------------	--------------------------------	--	------------------------------------	-------------------	--